

# PALEOCLIMATE ANALYSES OF DENSITY-DEPENDENT GROUNDWATER FLOW WITH PSEUDO-PERMAFROST IN DISCRETELY FRACTURED CRYSTALLINE ROCK SETTINGS

**Stefano D. Normani and Jonathan F. Sykes**

Department of Civil and Environmental Engineering  
University of Waterloo, Waterloo, Ontario, Canada, N2L 3G1  
e-mail: sdnorman@uwaterloo.ca, web page: <http://www.civil.uwaterloo.ca>

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**Summary.** A high resolution sub-regional scale (104 km<sup>2</sup>) density-dependent, groundwater flow model with fracture zone networks, hydromechanical coupling and pseudo-permafrost, was developed from a larger 5734 km<sup>2</sup> regional scale groundwater flow model of a Canadian Shield setting in fractured crystalline rock. The objective of the work is to examine the impact of glaciation and deglaciation on aspects of regional and sub-regional groundwater flow that are relevant to the long-term performance of a hypothetical nuclear fuel repository. The discrete fracture dual continuum numerical model FRAC3DVS-OPG was used for all simulations. A discrete fracture zone network model delineated from surface features was superimposed onto an 789887 element flow domain mesh. Orthogonal fracture faces (between adjacent finite element grid blocks) were used to best represent the irregular discrete fracture zone network. The crystalline rock between these structural discontinuities was assigned hydraulic and mechanical properties characteristic of those reported for the Canadian Shield at the Underground Research Laboratory at Pinawa, Manitoba.

The multiple 121000 year North American continental scale paleoclimate simulations are provided by W.R. Peltier using the University of Toronto Glacial Systems Model, whereby values of ice sheet normal stress, and proglacial lake depth are applied to the sub-regional model as surface boundary conditions. Thermal conditions resulting in permafrost formation are applied as a permeability reduction to both three-dimensional grid blocks and fractures that lie within the time varying permafrost zone. Density-dependent flow is required due to the presence of pore fluids deep in the Canadian Shield with densities of up to 1200 kg/m<sup>3</sup> and total dissolved solids concentrations in excess of 300 g/L. Hydromechanical coupling between the rock matrix and the pore fluid, due to the ice sheet normal stress, is included in the simulations. The flow model includes vertical strain and assumes that areal loads are homogeneous. The importance of hydromechanical coupling is shown using the sub-regional model to investigate the effect on the depth of glacial meltwater migration into the subsurface.

## 1 INTRODUCTION

Over the past 900000 years, the Canadian Shield has experienced approximately 9 episodes of complete glaciation. During the glaciation cycles, the entire Canadian land mass has been covered by a series of continental ice-sheets whose maximum thickness reached 4 km.<sup>1</sup>

Climate change and glaciation are not only a concern for the Canadian nuclear fuel waste disposal concept, but also for the Swedish<sup>2,3</sup> and Finnish concepts.<sup>4</sup> Peltier<sup>1</sup> and Marshall et al.<sup>5</sup> have developed glaciological reconstructions of the Laurentide Ice-Sheet using numerical models. During ice-sheet advance, a repository site would evolve from periglacial to subglacial with the site eventually overlain by up to 4 km of ice. Permafrost several hundred metres thick develops in advance of the ice-sheet.<sup>1</sup>

## 2 HYDROMECHANICAL COUPLING

One-dimensional vertical loading and unloading due to glaciation, erosion, or deposition, is a common simplification that can be applied in hydromechanical coupling; this coupling is applied to porous media, ignoring mechanical effects on fractures.<sup>6,7,8</sup> Assuming that the porous media, solid grains, and pore fluid are all compressible, the specific storage  $S_s$  is defined as:

$$S_s = \rho g \left[ \left( \frac{1}{K} - \frac{1}{K_s} \right) (1 - \lambda) + n \left( \frac{1}{K_f} - \frac{1}{K_s} \right) \right], \quad \lambda = \frac{2\alpha(1 - 2\nu)}{3(1 - \nu)}, \quad \alpha = 1 - \frac{K}{K_s} \quad (1)$$

where  $\rho$  is the fluid density [M/L<sup>3</sup>];  $g$  is the gravitational constant [L/T<sup>2</sup>];  $K$  is the drained bulk modulus of the porous media [M/T<sup>2</sup>L];  $K_s$  is the bulk modulus of the solids in the porous media [M/T<sup>2</sup>L];  $K_f$  is the bulk modulus of the pore fluid [M/T<sup>2</sup>L];  $\alpha$  is the Biot coefficient [/]; and  $\nu$  is the Poisson's ratio [/]. The bulk modulus  $K$  is defined as the reciprocal of compressibility, therefore  $K = 1/\beta$  as presented in Jaeger et al.<sup>8</sup>

The effect of mechanically loading the surface of a porous media is to transfer the load to both the porous media, and the pore fluid; the effects of mechanical loading and pore pressure affect each other, and are thus coupled. The groundwater flow equation can be modified to account for one-dimensional hydromechanical coupling as follows:

$$\frac{\partial}{\partial x_i} \left[ K_{ij} \left( \frac{\partial h}{\partial x_j} + \rho_r \frac{\partial z}{\partial x_j} \right) \right] \pm Q = S_s \frac{\partial h}{\partial t} - \frac{S_s \zeta}{\rho g} \frac{\partial \sigma_{zz}}{\partial t} \quad i, j = 1, 2, 3 \quad (2)$$

where  $K_{ij}$  is the porous media hydraulic conductivity tensor [L/T];  $h$  is the freshwater head [L];  $\rho_r$  is the relative fluid density [/];  $g$  is the gravitational constant [L/T<sup>2</sup>];  $z$  is the fluid elevation [L];  $\zeta$  is the one-dimensional loading efficiency [/]; and  $\sigma_{zz}$  is the vertical stress [M/LT<sup>2</sup>]. The loading efficiency is further defined as<sup>6,7</sup>:

$$\zeta = \frac{B(1 + \nu)}{3(1 - \nu) - 2\alpha B(1 - 2\nu)}, \quad B = \frac{\left( \frac{1}{K} - \frac{1}{K_s} \right)}{\left( \frac{1}{K} - \frac{1}{K_s} \right) + n \left( \frac{1}{K_f} - \frac{1}{K_s} \right)} \quad (3)$$

where  $B$  is the Skempton coefficient and physically represents the ratio of change in fluid pressure to a change in mean effective stress under undrained conditions.<sup>7</sup>

### 3 SUB-REGIONAL MODEL

Both steady-state and pseudo steady-state simulations need to be run prior to paleoclimate simulations. For a given modelling scenario, a steady-state simulation without total dissolved solids (TDS) is run using FRAC3DVS-OPG. The resulting freshwater heads from the steady-state simulation are modified to account for the presence of high TDS, where the development of the initial TDS distribution is described in §3.2. The adjusted freshwater heads and initial TDS distribution form the initial conditions for a 1 Ma transient simulation, whose final state at 1 Ma represents a pseudo steady-state. The resulting pseudo steady-state freshwater heads and TDS distribution are used as the initial conditions for the paleoclimate simulations.

#### 3.1 Spatial Discretization

The three-dimensional sub-regional domain was discretized into 847080 nodes, and 789887 brick elements, covering an area of 104 km<sup>2</sup>. The grid is orthogonal with each equivalent porous medium (EPM) block having the same planimetric dimensions of 50 m by 50 m and was vertically discretized into 19 layers. The top of model layer 19 is defined by a digital elevation model (DEM) using the linear interpolation method within the Spatial Analyst package of ArcView 3.2a. Layers 1 through 14 inclusive have a constant thickness, while Layers 15 through 19 have a variable thickness which depends on the elevation of surface topography.

#### 3.2 Initial Conditions

The paleoclimate simulations require coupled density-dependent flow and transport due to the high TDS concentrations found deep in the Canadian Shield. An initial TDS distribution is required for the pseudo steady-state model. An equation for TDS versus depth  $d$  for groundwaters from the Canadian Shield, based on Figure 2b in Frappe and Fritz<sup>9</sup>, is presented as Equation (4) where  $TDS$  is in units of g/L.

$$TDS = \begin{cases} 10^{0.001981697d}, & \text{for } d \leq 1250 \text{ m;} \\ 300, & \text{for } d > 1250 \text{ m.} \end{cases} \quad (4)$$

Setting an upper bound for the TDS distribution as an initial condition allows recharge waters to reduce the TDS concentrations in fractures, and in the nearby rock matrix during the pseudo steady-state simulations, providing a more representative initial condition for the paleoclimate simulations.

#### 3.3 Boundary Conditions

For the steady-state and pseudo steady-state models, specified head is applied to surface nodes associated with water features, while a recharge rate of 1.0 mm/a is applied elsewhere

across the top of the model. A zero-flux boundary condition for flow is applied to the model's sides and bottom. For the paleoclimate simulations, a tracer associated with water entering the groundwater model from the top surface is used to investigate glacial meltwater migration in the subsurface during and subsequent to a glaciation and deglaciation episode. A Cauchy boundary condition at the model's top surface is used with a tracer concentration of unity.

A plot of various NN2778 Glacial Systems Model (GSM) model outputs, including ice thickness and permafrost depth, for the grid cell containing the sub-regional modelling domain are shown in Figure 1. The ice thickness, lake depth, and permafrost depth outputs are applied to the paleoclimate groundwater flow simulations.

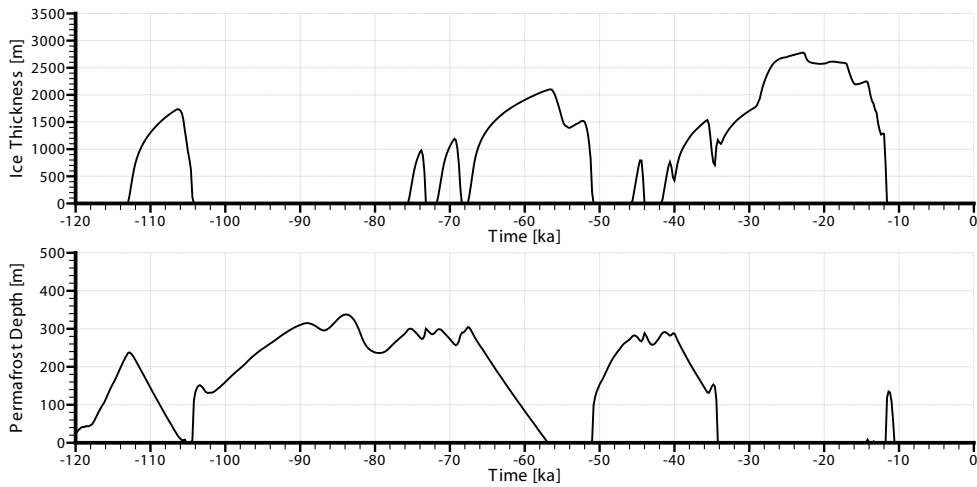


Figure 1: Plots of ice thickness and permafrost depth versus time for the nn2778 GSM grid block containing the sub-regional modelling domain, provided by Peltier<sup>10</sup>

### 3.4 Fracture Network Model

Srivastava<sup>11</sup> developed a framework for generating equally likely fracture network models. The first unconditioned fracture network model (FNM) from Srivastava<sup>12</sup> was chosen for all paleoclimate simulations. The FNM was then mapped onto an orthogonal brick element mesh whereby planar quadrilateral elements are used to represent fracture zones of higher permeability. The resulting orthogonal FNM is shown in Figure 2.

### 3.5 Properties

Properties for the paleoclimate simulations are provided in Table 1. The values for Young's Modulus  $E$  and Poisson's ratio  $\nu$  are provided in Chan and Stanchell<sup>13</sup>. The calculation of specific storage  $S_s$  and one-dimensional loading efficiency  $\zeta$  assume the Biot coefficient  $\alpha = 0.73$ <sup>13</sup>, and a fluid compressibility of  $4.4 \times 10^{-10} \text{ Pa}^{-1}$ . Fracture properties for solute transport include a longitudinal dispersivity of 250 m, and a transverse dispersivity of 25 m. Dispersivities

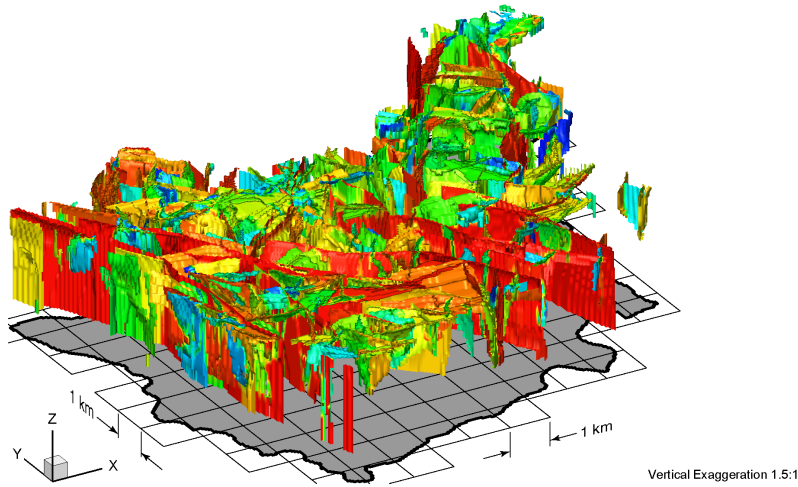


Figure 2: 3-D perspective view of orthogonal FNM

for the matrix include a longitudinal dispersivity of 50 m, a transverse horizontal dispersivity of 5 m and a transverse vertical dispersivity of 0.5 m.

Fracture zone hydraulic conductivity decreases with depth from a value of approximately  $1 \times 10^{-13}$  m/s at surface to  $1 \times 10^{-16}$  m/s at depths of 1500 m. The permafrost depths were used to select any FRAC3DVS-OPG grid block whose top face was within the permafrost zone for each time step. A permafrost hydraulic conductivity of  $5 \times 10^{-11}$  m/s was applied and assumed to be valid for fractured rock systems, even though McCauley et al.<sup>14</sup> performed experiments with frozen soils. Any fracture which is adjacent to a hexahedral element whose permeability has been reduced due to the presence of permafrost, is itself subject to the same permeability reduction.

#### 4 PALEOCLIMATE SIMULATIONS

A steady-state simulation was performed, followed by a pseudo steady-state density-dependent simulation which was used as the initial conditions for the paleoclimate simulations. The purpose of pseudo steady-state density-dependent simulations is to allow the flow system to equilibrate from the initial TDS distribution. Solute free recharge water tends to flush fracture zones and reduce TDS in the adjacent matrix blocks.

The first paleoclimate simulation uses the parameters listed in Table 1, and represents the base-case analysis. Scenario 1 uses the NN2778 glaciation scenario, with decreasing fracture zone permeability with depth, a computed one-dimensional loading efficiency, and coupled density-dependent flow and transport. Scenario 2 is identical to Scenario 1, but applies a one-dimensional loading efficiency  $\zeta = 0$  to determine the sensitivity of glacial water infiltration to the loading efficiency term in the groundwater flow equation.

Layer	Depth Range [m]	$k_H$ [m <sup>2</sup> ]	$k_V$ [m <sup>2</sup> ]	$n$ [/]	TDS [g/L]	$\rho$ [kg/m <sup>3</sup> ]	$E$ [GPa]	$\nu$ [/]	$S_s$ [m <sup>-1</sup> ]	$\zeta$ [/]
19	0–10	$1.0 \times 10^{-13}$	$1.0 \times 10^{-12}$	0.003	1.02	1000.7	20.0	0.25	$3.76 \times 10^{-7}$	0.80
18	10–30	$1.0 \times 10^{-14}$	$1.0 \times 10^{-13}$	0.003	1.10	1000.7	20.0	0.25	$3.76 \times 10^{-7}$	0.80
17	30–70	$9.5 \times 10^{-16}$	$9.5 \times 10^{-15}$	0.003	1.26	1000.8	20.0	0.25	$3.76 \times 10^{-7}$	0.80
16	70–150	$2.7 \times 10^{-16}$	$2.7 \times 10^{-15}$	0.003	1.68	1001.1	20.0	0.25	$3.76 \times 10^{-7}$	0.80
15	150–250	$5.6 \times 10^{-17}$	$5.6 \times 10^{-16}$	0.003	2.56	1001.7	30.0	0.25	$2.55 \times 10^{-7}$	0.78
14	250–350	$1.4 \times 10^{-17}$	$1.4 \times 10^{-16}$	0.003	4.03	1002.7	30.0	0.25	$2.56 \times 10^{-7}$	0.78
13	350–450	$4.7 \times 10^{-18}$	$4.7 \times 10^{-18}$	0.003	6.37	1004.2	60.0	0.25	$1.34 \times 10^{-7}$	0.75
12	450–550	$2.0 \times 10^{-18}$	$2.0 \times 10^{-18}$	0.003	10.05	1006.7	60.0	0.25	$1.35 \times 10^{-7}$	0.75
11	550–625	$1.1 \times 10^{-18}$	$1.1 \times 10^{-18}$	0.003	14.81	1009.9	60.0	0.25	$1.35 \times 10^{-7}$	0.75
10	625–675	$8.0 \times 10^{-19}$	$8.0 \times 10^{-19}$	0.003	19.54	1013.0	60.0	0.25	$1.36 \times 10^{-7}$	0.75
9	675–725	$6.3 \times 10^{-19}$	$6.3 \times 10^{-19}$	0.003	24.55	1016.4	60.0	0.25	$1.36 \times 10^{-7}$	0.75
8	725–775	$5.1 \times 10^{-19}$	$5.1 \times 10^{-19}$	0.003	30.84	1020.6	60.0	0.25	$1.37 \times 10^{-7}$	0.75
7	775–825	$4.2 \times 10^{-19}$	$4.2 \times 10^{-19}$	0.003	38.74	1025.8	60.0	0.25	$1.37 \times 10^{-7}$	0.75
6	825–875	$3.6 \times 10^{-19}$	$3.6 \times 10^{-19}$	0.003	48.67	1032.4	60.0	0.25	$1.38 \times 10^{-7}$	0.75
5	875–950	$3.0 \times 10^{-19}$	$3.0 \times 10^{-19}$	0.003	65.26	1043.5	60.0	0.25	$1.40 \times 10^{-7}$	0.75
4	950–1050	$2.4 \times 10^{-19}$	$2.4 \times 10^{-19}$	0.003	98.38	1065.6	60.0	0.25	$1.43 \times 10^{-7}$	0.75
3	1050–1200	$1.9 \times 10^{-19}$	$1.9 \times 10^{-19}$	0.003	179.62	1119.7	60.0	0.25	$1.50 \times 10^{-7}$	0.75
2	1200–1400	$1.5 \times 10^{-19}$	$1.5 \times 10^{-19}$	0.003	269.40	1179.6	60.0	0.25	$1.58 \times 10^{-7}$	0.75
1	1400–1600	$1.3 \times 10^{-19}$	$1.3 \times 10^{-19}$	0.003	300.00	1200.0	60.0	0.25	$1.61 \times 10^{-7}$	0.75

Table 1: Sub-regional model properties for paleoclimate simulations with density-dependent coupling

#### 4.1 Simulation Results

A tracer of unit concentration is applied as a Cauchy boundary condition to all inflow surface nodes at the beginning of the paleoclimate simulation. This tracer is used to characterize the migration, from the surface, of recharge water that occurs during the paleoclimate simulation; the recharge water includes glacial meltwater. The depth of glacial meltwater penetration into the subsurface is of interest because dissolved oxygen ( $O_2$ ) can be transported to great depths by infiltrating meltwater, and is a consideration in repository siting. Tracer concentrations for the modelling domain are presented in Figure 3 at the end of the 120 ka simulation; the tracer is predominantly in the matrix with some tracer appearing in fractures at mid-depth.

Scenario 2 investigates the role of hydromechanical coupling, by setting the one-dimensional loading efficiency,  $\zeta = 0$ . The tracer, as shown in Figure 4, has migrated deeper into the subsurface as compared to Scenario 1, due to the increased vertical gradients resulting from ignoring hydromechanical coupling.

The differences between Figure 3 and Figure 4 demonstrate that hydromechanical coupling is an important process that must be considered during long-term groundwater simulations under paleoclimate conditions.

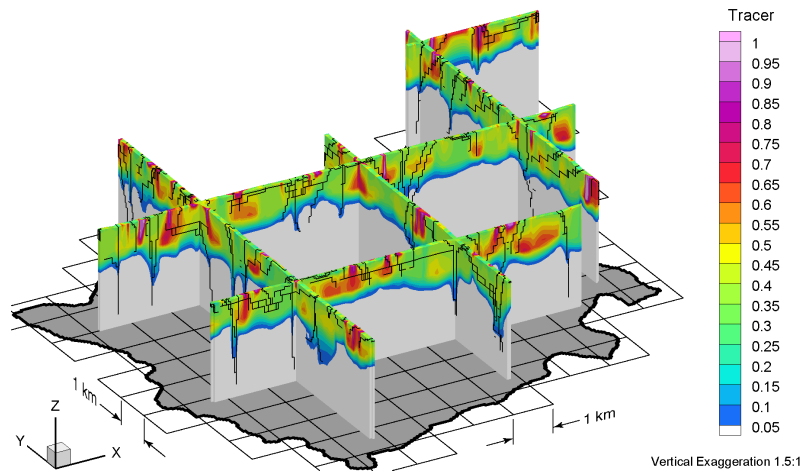


Figure 3: Migration of unit tracer during 120 ka paleoclimate simulation for computed one-dimensional loading efficiency

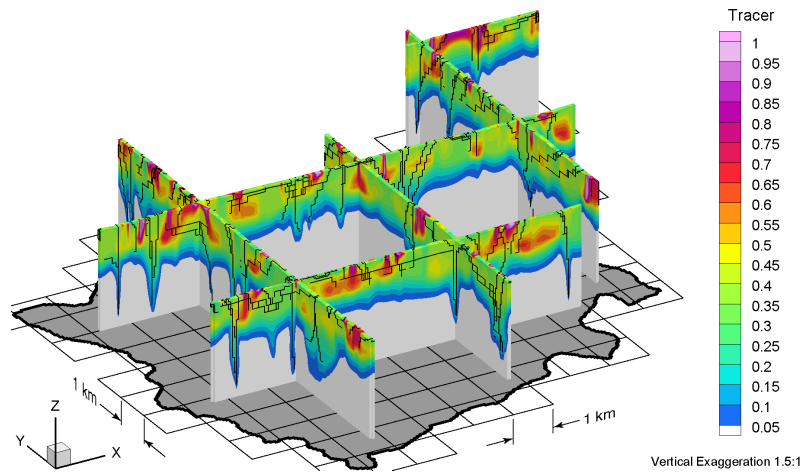


Figure 4: Migration of unit tracer during 120 ka paleoclimate simulation for one-dimensional loading efficiency  $\zeta = 0$

## 5 SUMMARY AND CONCLUSIONS

The choice of hydromechanical coupling has significant influences on unit tracer migration. The unit tracer migrates deeper into the subsurface when ignoring hydromechanical coupling while assuming the hydraulic boundary condition at ground surface is equal to the pressure at the base of an ice-sheet. Hydromechanical coupling is a very important mechanism which affects hydraulic gradients during a glaciation event. Groundwater flow models which do not include a suitable form of hydromechanical coupling, one-dimensional or otherwise, must be used with caution as very large vertical gradients can be generated, resulting in higher pore water velocities, and enhanced migration of surface waters into the subsurface environment.

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